

PO Box 215 Bondi NSW 2026 | ph.: +61 2 9332 2024 | fax.: +61 2 9332 2022 | mob.: +61 (0)4 1497 8067 | email: o.s@tefconsult.com.au | www.tefconsult.com.au

AN AMENDED TRAFFIC AND PARKING IMPACTS REPORT FOR A DEVELOPMENT APPLICATION FOR A PROPOSED AFFORDABLE HOUSING DEVELOPMENT AT NO. 30-38 IRONBARK AVENUE, CASULA NSW 2170

Property address	30-38 Ironbark Avenue, Casula NSW 2170
Client	SGCH
Prepared by	O. Sannikov, MEngSc (Traffic Engineering), MIEAust, PEng, FAITPM
Date	02/07/2019
Job No.	18052
Report No.	18052 Rep 02 (updated on-street parking analysis)

ltem	Report
Site location	• Refer to Figure 1.
Existing land	Five (5) single storey residential dwellings
use	
Proposed	Affordable housing development comprising
development	 63 residential apartments in total including
	 15 one bedroom units
	 48 two bedroom units
	• Ground level car park
	 30 car parking spaces including
	• 7 car spaces for people with disabilities





Figure 1. Site location.



ltem	Report								
	Existing	g traff	ic and parking situation						
Street	•	Refe	er to Figure 2.						
characteristics	٠	The	main roads bounding the proposed development are described below.						
		o	Ironbark Avenue						
			Local Road						
			 2 travel lanes and parking opportunities on both sides of the street 						
		0	South Western Motorway						
			 State Road (MR 6005) 						
			 6 travel lanes and no parking lanes 						
		0	Kurrajong Road						
			 Regional Road (MR 7263) 						
			 2 travel lanes and no parking lanes 						
		0	Tallowwood Avenue						
			Local road						
			 2 travel lanes and parking opportunities on both sides of the street 						
		0	Ingham Drive						
			Collector Road						
			 2 travel lanes and parking opportunities on both sides of the street 						
		o	Other streets in the surrounding area are local/local collector roads. Street conditions are typical for a residential area, with low to moderate traffic volumes.						
			 General speed limit is 50 km/h on local streets around the site. 						
On-street parking	•	On-s Cres	street parking is available on Ironbark Avenue and on nearby streets such as Jacaranda scent, Sycamore Avenue, Tallowwood Avenue and Brigalow Avenue.						
availability		0	There are unrestricted car parking opportunities across all streets.						
	Public Transport								
Bus	•	The	site is located approximately 350 metres from bus stops along Kurrajong Road.						
	٠	Refe	er to Figure 3.						
		۰	Bus Route 851						
			 Liverpool to Carnes Hill Marketplace via Cowpasture Rd 						
			• 2 services operate during the morning peak and services operate approximately every 30 minutes during the afternoon peak.						
			 Carnes Hill Marketplace to Liverpool via Cowpasture Rd 						
			• 4 services operate during both morning and afternoon peaks.						
		٥	Bus Route 852						
			 Liverpool to Carnes Hill Marketplace via Greenway Dr & Cowpasture Rd 						
			5 services operate during the morning peak in both directions						
			• Services operates approximately every 30 minutes during the afternoon peak in both directions.						
		٥	Bus Route 865						
			Liverpool to Casula via Lurnea Shops						
			• Services operate approximately every 30-40 minutes during both morning and afternoon peaks in both directions.						
		٥	Bus Route 866						
			Liverpool to Casula						

• Services operate approximately every 30-40 minutes during both morning and afternoon peaks in both directions.





Figure 2. Street characteristics.





Figure 3. Public transport - bus stops.



Item	eport										
	Surveys and survey results										
Parking survey	 A parking demand survey was conduct 11th of April 2018 	ted on Thursday the 10 th of May 2018 and Friday the									
	 The morning survey was between 	6:30 a.m. and 9:30 a.m.									
	 The afternoon survey was between 	en 3:00 p.m. and 7:00 p.m.									
	• Refer to Figure 4 for survey locations										
	 Areas in red represent a walking of 	listance of up to 150 metres from the site location									
	 Areas in blue represent a walking 	distance of 150 - 250 metres from the site location.									
	• The following analysis now conserva parking on both sides of the street is the report. The result still demonstrat	tively assumes that only 70% of the full kerb length available as opposed to 100% in the first version of e ample parking opportunities as detailed below.									
Survey results	• Refer to Table 1 for survey results										
	• Area 1a-1b (Ironbark Ave on the same	side as the proposed development)									
	 The morning peak occurred from 	6:30 a.m. to 7:00 a.m.									
	• The afternoon peak occurred at 3	:00 p.m. and from 4:00 p.m. to 5:30 p.m.									
	 The survey results indicated that day (to a maximum of 16) in the s 	there were at least 14 spaces vacant throughout the urvey area.									
	• Area 1c-1e (Ironbark Ave on the oppo	site side of the proposed development)									
	 The morning peak occurred from a.m. 	n 6:30 a.m. to 7:00 a.m. and from 8:00 a.m. to 9:00									
	• The afternoon peak occurred at 3	:00 p.m. and from 4:30 p.m. to 5:00 p.m.									
	 The survey results indicated that day (to a maximum of 11) in the s 	there were at least 8 spaces vacant throughout the urvey area.									
	Area 2a-6 (all other areas)										
	 The morning peak occurred from 	8:00 a.m. to 9:30 a.m.									
	• The afternoon peak occurred at 7	:00 p.m.									
	 The survey results indicated that day (to a maximum of 62) in the s 	there were at least 54 spaces vacant throughout the urvey area.									

• There are ample parking opportunities in the surrounding streets.





Figure 4. Parking survey locations.



Table 1. Parking survey results.

	Number of parked cars														
	Parking Location												Total		
Time	1a	1b	1c	1d	1e	2a	2b	3a	3b	4	5	6	1a-1b	1c-1e	2a-6
6:30	3	3	2	3	0	0	2	1	0	3	2	0	6	5	8
7:00	3	3	2	3	0	0	2	0	0	2	2	0	6	5	6
7:30	2	3	1	3	0	0	2	0	1	2	2	0	5	4	7
8:00	2	3	2	3	0	0	2	0	2	3	2	0	5	5	9
8:30	2	3	2	3	0	0	2	0	2	3	2	0	5	5	9
9:00	2	3	2	3	0	0	2	0	2	3	2	0	5	5	9
9:30	2	3	2	1	0	0	2	1	1	3	2	0	5	3	9
15:00	3	3	1	4	1	0	0	0	0	2	1	1	6	6	4
15:30	3	1	1	3	0	0	0	0	0	2	1	0	4	4	3
16:00	3	3	1	4	0	0	3	0	0	1	1	0	6	5	5
16:30	3	3	1	5	0	0	3	0	0	2	1	1	6	6	7
17:00	3	3	1	5	0	0	3	0	0	3	1	1	6	6	8
17:30	3	3	1	3	0	0	1	0	0	3	2	0	6	4	6
18:00	3	2	1	3	0	0	2	0	0	4	2	0	5	4	8
18:30	3	2	1	4	0	0	2	0	0	4	2	1	5	5	9
19:00	3	2	1	4	0	0	2	0	0	4	4	1	5	5	11
No of spaces	6	14	3	6	5	9	8	4	4	16	14	11	20	14	65

	Number of vacant parking spaces														
	Parking Location												Total		
Time	1a	1b	1c	1d	1e	2a	2b	3a	3b	4	5	6	1a-1b	1c-1e	2a-6
6:30	3	11	1	3	5	9	6	3	4	13	12	11	14	9	57
7:00	3	11	1	3	5	9	6	4	4	14	12	11	14	9	59
7:30	4	11	2	3	5	9	6	4	3	14	12	11	15	10	58
8:00	4	11	1	3	5	9	6	4	2	13	12	11	15	9	56
8:30	4	11	1	3	5	9	6	4	2	13	12	11	15	9	56
9:00	4	11	1	3	5	9	6	4	2	13	12	11	15	9	56
9:30	4	11	1	5	5	9	6	3	3	13	12	11	15	11	56
15:00	3	11	2	2	4	9	8	4	4	14	13	10	14	8	61
15:30	3	13	2	3	5	9	8	4	4	14	13	11	16	10	62
16:00	3	11	2	2	5	9	5	4	4	15	13	11	14	9	60
16:30	3	11	2	1	5	9	5	4	4	14	13	10	14	8	58
17:00	3	11	2	1	5	9	5	4	4	13	13	10	14	8	57
17:30	3	11	2	3	5	9	7	4	4	13	12	11	14	10	59
18:00	3	12	2	3	5	9	6	4	4	12	12	11	15	10	57
18:30	3	12	2	2	5	9	6	4	4	12	12	10	15	9	56
19:00	3	12	2	2	5	9	6	4	4	12	10	10	15	9	54



Item	Report								
	Traffic co	ounts							
Intersection	Location	/ type of co	ntrol Ironbark Avenue / Tallo	Ironbark Avenue / Tallowwood Avenue (T-intersection)					
traffic volume			Tallowwood Avenue / K	Tallowwood Avenue / Kurrajong Road (T-intersection)					
counto			Kurrajong Road / Ingha	m Drive (roundabout)					
	Date / Da	ay of the we	ek Thursday 10 th May 2018	3 (PM) to Friday 11 th May 2018 (AM)					
	Time per	iod (AM)	06:30 to 09:30; peak ho	our occurred at 08:15-09:15					
	Time per	iod (PM)	15:00 to 19:00; peak ho	our occurred at 16:45-17:45					
	•	Refer to Fig	ure 5.						
Intersection	•	SIDRA Inters	section 7.0 software was used t	o check the intersection performance					
operation	٠	Results of th	ne SIDRA modelling determined	I the following:					
	 Ironbark Avenue / Tallowwood Avenue intersection operates at a good Level of Service (LoS A) during both the morning and afternoon peak hours. 								
		 Tallowwood Avenue / Kurrajong Road intersection operates at a good Level of Service (LoS A) during the morning peak hour and at a good/satisfactory Level of Service (LoS B) during the afternoon peak hour. 							
	 Kurrajong Road / Ingham Drive intersection operates at a good Level of Service (LoS , during both the morning and afternoon peak hours. 								
		• refer to	Appendix A for detailed result	ts					
		• refer to	RMS (RTA) Level of Service det	finitions below					
			Level of service criter	ia for intersections					
	Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs					
	Α	< 14	Good operation	Good operation					
	В	15 to 28	Good with acceptable delays & space capacity	are Acceptable delays & spare capacity					
	С	29 to 42	Satisfactory	Satisfactory, but accident study required					
	D	43 to 56	Operating near capacity	Near capacity & accident study required					
	E	57 to 70	At capacity; at signals, incidents w excessive delays; Roundabouts re other control mode	ill cause At capacity, requires other control mode quire					

Source: RTA (2002) Guide to Traffic Generating Developments





Figure 5a. Existing Traffic Volumes - morning peak





Figure 5b. Existing Traffic Volumes - afternoon peak



ltem	Report									
Planning control document 1	State Environmental Planning Policy (Affordable Rental Housing) 2009 (AHSEPP 2009)									
	Division 1 – In-fill affordable housing									
	Requirement Compliance									
	SEPP (Affordable Rental Housing) 2009 (AHSEPP)									
	The proposed development is classified under Division 1 In-fill affordable housing									
	Clause 10 Development to which Division applies									
	i. This Division applies to development for the purposes of dual occupancies, multi dwelling housing or residential flat buildings if:									
	(a) the development concerned is permitted with consent under another environmental planning instrument, and									
	(b) the development is on land that does not contain a heritage item that is identified in an environmental planning instrument, or an interim heritage order or on the State Heritage Register under the Heritage Act 1977.									
	ii. Despite subclause (1), this Division does not apply to development on land in the Sydney region unless all or part of the development is within an accessible area.									
	iii. Despite subclause (1), this Division does not apply to development on land that is not in the Sydney region unless all or part of the development is within 400 metres walking distance of land within Zone B2 Local Centre or Zone B4 Mixed Use, or within a land use zone that is equivalent to any of those zones.									
	Clause 14 Standards that cannot be used to refuse consent									
	(2) General									
	A consent authority must not refuse consent to development to which this Division applies on any of the following grounds:									
	 (i) in the case of a development application Complies made by a social housing provider for development on land in an accessible area—at least 0.4 parking spaces are provided for each dwelling containing 1 bedroom, at least 0.5 parking spaces are provided for each dwelling containing 2 bedrooms and at least 1 parking space is provided for each dwelling containing 3 or more bedrooms 									
	The applicant SGCH is a registered Tier 1 Community Housing Provider (social housing provider).									
	Furthermore, the proposed development is within 400 metres walking distance of a bus stop used by a regular bus service (within the meaning of the Passenger Transport Act 1990) and is therefore in an accessible area.									
	Car parking required: Car parking provided:									
	0.4 spaces per 1 bedroom Total of 30 spaces									
	• 0.4 x 15 = 6 spaces Complies with Division 1 AHSEPP requirements									
	0.5 spaces per 2 bedroom									
	• 0.5 x 48 = 24 spaces									
	Total parking spaces required									
	• 6 + 24 = 30 spaces									



Item	Report												
Planning	Liverpool Development Control Plan 2008												
control document	 Part 1 – General controls for all development 												
document	Liverpool Local Environmental Plan 2008												
Building design (car parking)													
	Requirement Compliance												
	Section 20. Car Parking and Access												
	20.1 Overall Design Considerations												
	The layout of a car parking area shall consider the entire facility, including car parking modules, landscaping, circulation aisles and roadways, access driveways and, if necessary, frontage road access as an integrated coordinated design. The management of traffic within a car parking facility should take into account:												
	 The need for traffic to move to and from Complies the frontage road with minimum disruption to passing traffic and maximum pedestrian safety. 												
	 Provision of adequate capacity in Complies circulation roadways and aisles to handle peak hour movements without congestion. 												
	 Avoid as far as practicable conflicts Complies between intersecting streams of circulating traffic. 												
	 Minimum length travel paths between Complies entry/exit points and car parking spaces. 												
	20.2 Car Parking Provision and Service Facilities by Land Use												
	1. Tables 12 and 13 outline the number of car Refer to the previous section, State parking spaces and any other facilities required Environmental Planning Policy (Affordable Renta for the accommodation of vehicles on site for Housing) 2009 (AHSEPP 2009) as it overrides DCF each land use type. In proposals where requirements for car parking rates and calculations of car parking requirements result in calculations. fractions of spaces being required, the fraction will be rounded up to the nearest whole space. Where developments comprise separately defined facilities, for example a hotel with a restaurant; the relevant requirements of each facility must be satisfied.												

20.3 Car Parking Design

Car space dimensions (refer to Table 14 below) Complies

Table 14. Car space dimensions of off-street car parking bays at 90°

Land use types	Width	Length 1	Length 2	Aisle Width
Tenant, employee and commuter car parking, universities (generally all day car parking)	2.4m	5.4m	4.8m	6.2m
Long-term city and town centre car parking, sport facilities, entertainment centres, hotels, motels, airport visitors (generally medium term car parking)	2.5m	5.4m	4.8m	5.8m
Short-term city and town centre car parking, shopping centres, department stores, supermarkets, hospitals and medical centres (generally short term car parking and where children and goods can be expected to be loaded into vehicles)	2.6m	5.4m	4.8m	5.8m
Car parking for people with disabilities (see next section)	3.2m	5.4m	4.8m	5.8m



Item	Report	
	Requirement	Compliance
	20.4 Internal Driveways	
	Gradient	
	1. Driveways are to be in accordance with the relevant Australian Standard. The maximum change in gradient is to be as shown in the "Maximum Gradients of Internal Driveway" diagram (See Figure 3).	Complies
	2. Measured parallel to the angle of car parking 1 in 20 (5%); and	Complies
	3. Measured at 90° to the angle of car parking – 1 in 16 (6.25%).	Complies
	Widths	
	1. For internal driveways between the access driveway and the car parking area the minimum carriageway width depends on the number of car parking spaces and service bays served.	Complies
	2. Consideration should be given to increase these widths where high levels of heavy vehicles usage are anticipated.	Complies
	3. By definition circulation driveways should not have car parking on them.	Complies
	4. The minimum internal driveway widths are to be provided in accordance with Table 4.	Complies

Table 15. Minimum internal driveway widths

Table 15 Internal driveway widths

		Number of Car Parking Spaces / Service Bays							
_		1 - 15 spaces and length not exceeding 40m	spaces	Over 40 sp	aces				
	Width	3.5m		5m	. 6	- 6.5m			
Desig	'n								
1. Loc can b	ate and e observ	design car-parking areas so ed by adjoining uses.	they	Complies					
2. Mii vehic are in nearb	nimise th ular entr close pr by active	e number of pedestrian and y and exit points, and ensur oximity to each other and to uses.	Complies						
3. Sta and s	ff car pai ecured.	rking areas should be separa	ated	Complies					
4. Pro came possil	ovide surv ras or de ble.	veillance measures such as s vices and security guards w	ecurity here	Complies					
8. Pedestrian pathways should be integrated into the design and allow for maximum safety, especially for people with a disability and people using prams. Pathways should be clearly marked and well lit.				Complies					
9. Inte speed	ernal driv l environ	veway should be designed for ment.	or a low	Complies					
20.5 I	Driveway	/s Crossings							
Locat	ion of Dı	riveway Crossings							



Item	Report	
	Requirement	Compliance
	 Driveway Crossings shall be located a minimum distance from the following items: 0.5m from all drainage structures on the kerb and gutter; 1.0m from side property boundaries; 6m from a kerb tangent point of a street corner. 	Complies
	2. Driveway Crossings should where possible avoid the need to remove existing street trees.	Complies
	3. Driveway Crossings should where possible avoid changes to existing public utility infrastructure including drainage and any relocation of such shall be the development's expense.	Complies
	4. Where a development site has frontage to a Classified Road, the Driveway Crossings should be located on an alternative street.	Driveway located on Ironbark Avenue Complies
	5. Where a Driveway Crossing is proposed directly from a Classified Road, a deceleration lane may be required.	Not applicable
	6. Locate the entrance at the first Driveway Crossing from the adjacent kerbside lane.	Complies
	7. Avoid a driveway layout, which may result in on-street queuing.	Complies
	8. All vehicles must enter and leave the property in a forward direction (except in the case of dwelling houses and Attached dwellings and Semi detached dwellings)	Complies
	9. Locate each Driveway Crossing so that it is clear of all obstructions, e.g. poles, trees, which may prevent drivers from having a timely view of pedestrians.	Complies
	Design of Driveway Crossings	
	1. Design each Driveway Crossing so that it is relatively level within 6m of the site boundary or any pedestrian way, the recommended maximum gradient is 5%.	Complies
	2. Signpost each Driveway Crossing with appropriate entry, exit and keep left signs.	Complies
	3. Decorative Driveway Crossings over the footpath area will only be permitted if it is compatible with the amenity of the locality.	Complies
	4. In business zones any Driveway Crossing shall be compatible with the existing and future paving pattern.	Complies
	Width of Driveway Crossings	
	1. Driveway crossing widths shall be in accordance with tables 5 and 6.	Complies
		-

Table 16 Car Parking Spaces served by the Driveway Type

Street Frontage	Number of Car Parking Spaces served by the Driveway Type												
	Less than 25	25-100	101-300	301-600	More than 600	Heavy Vehicles							
Major	1-2	2-3	3-4	4	5	7							
Minor	. 1 .	1-2	2-3	3-4	4	6							



Item Report Requirement Compliance 2. Major Street Frontage includes Classified Complies

2. Major Street Frontage includes Classified Roads and Sub Arterial Roads under Council's Road Hierarchy.

Table 17 Driveway crossing widths

Туре	Entry Width	Exit Width	Minimum separation of driveways	Splay at kerb line	Kerb return turnout radius
	w	w		S	R
1	3 – m	Combined	NA	0.5m	-
2	<mark>6 – 9</mark> m	Combined	NA	1m	-
3	6m	4 – 6 m	1 - 3m	1m	2 – 9m
4	6 – 8m	6 – 8 m	1 - 3m	1m	2 – 9m
5	Direct feed from a	controlled intersed	ction via a public stre	eet	
6	8 – 10m	8 –10m	3m	1m	2 – 9m
7	10 –12m	10 –12m	3m	1m	2 – 9m



Item	Report
	Traffic impacts
Traffic	Base traffic generation rates
generation	 From RMS (2002) Guide to Traffic Generating Developments
	 Updated statistics from TDT 2013 / 04a
	High density residential developments
	 AM peak – 0.19 trips per unit
	 26 % in and 74 % out
	• PM peak - 0.15 trips per unit
	 66% in and 34% out
	Existing traffic generation
	Dwelling houses
	 day peak hour vehicle trips = 0.99 per dwelling
	• 0.99×5 = 4.95 one way trips, say 5 one way trips
	Traffic generated by proposed development
	• Refer to Figure 6.
	 High density residential development
	Morning peak
	• 0.19 × 63 = 11.97, say 12 trips (in + out)
	• 12 × 26% = 3.11, say 3 trips in
	• 12 × 74% = 8.86, say 9 trips out
	Afternoon peak
	 0.15 × 63 = 9.45, say 9 trips (in+out)
	• 9 × 66% = 6.24, say 6 trips in
	• 9 × 34% = 3.21, say 3 trips out
Traffic distribution	• Trip generation and attraction is assumed to be equal in all directions, with trip distribution taking into account the surrounding street network, connections and turn restrictions.
	• Refer to Figures 6a and 6b.
SIDRA	Impact on intersection operation
modelling	 The operation of the street network under additional traffic loading was modelled using SIDRA Network modelling.
	• Table 2 contains a summary of SIDRA modelling results.
	• The modelling results were compared to the results of the existing traffic model. The results indicated the following:
	 The Level of Service for the Ironbark Avenue / Tallowwood Avenue intersection remained the same, LoS A for both the morning and afternoon peak hours.
	 The Level of Service for the Tallowwood Avenue / Kurrajong Road Avenue intersection remained the same, LoS A for the morning peak hour and LoS B for the afternoon peak hour.
	 The Level of Service for the Kurrajong Road / Ingham Drive intersection remained the same, LoS A for both the morning and afternoon peak hours.
	 Modelling results indicate very minor increases in average delays and queue lengths. There will be no noticeable changes to the existing road network operation.
Conclusion	• Additional traffic generation is very minor and will have no noticeable impact on the existing road network.
	 The operation of the intersection will remain unchanged.





Figure 6a. Distribution of additional traffic volumes - morning peak





Figure 6b. Distribution of additional traffic volumes - afternoon peak



Table 2. Results of SIDRA analysis

		Sidra													
Nº	Nº Intersection			AM							PM				
		AVD	LOS	DS	Queue, m	Mc	veme	ement AV		LOS	DS	Queue, m	Mo	oveme	nt
1	Kurrajong Rd / Ingham Dr	13.3	А	0.669	53.3	KRd	WB	Т	14.0	А	0.72	65.7	KRd	EB	Т
2	Kurrajong Rd / Tallowwood Ave	13.0	А	0.311	3.0	KRd	WB	Т	15.2	В	0.34	5.4	KRd	WB	Т
3	Tallowwood Ave / Ironbark Ave	5.5	А	0.022	0.5	IAve	EB	R	5.7	Α	0.03	0.6	IAve	EB	R
	Future, Commuter Peak														
		Sidra													

Nº	Intersection				AM			PM					
		AVD	LOS	DS	Queue, m	Movement	AVD	LOS	DS	Queue, m	Mo	veme	nt
1	Kurrajong Rd / Ingham Dr	13.3	А	0.672	54.1	KRd WB T	14.1	Α	0.73	66.3	KRd	EB	Т
2	Kurrajong Rd / Tallowwood Ave	13.1	А	0.313	3.3	KRd WB T	15.2	В	0.34	5.9	KRd	WB	Т
3	Tallowwood Ave / Ironbark Ave	5.5	А	0.03	0.7	IAve EB R	5.7	А	0.04	0.6	IAve	EB	R

Legend:			
KRd	Kurrajong Rd	EB	Eastbound
IDr	Ingham Dr	WB	Westbound
TAve	Tallowwood Ave	NB	Northbound
IAve	Ironbark Ave	SB	Southbound
		т	Through movement
		R	Right hand turn
		L	Left hand turn
L			

	Level of service criteria for intersections											
Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs									
А	< 14	Good operation	Good operation									
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity									
С	29 to 42	Satisfactory	Satisfactory, but accident study required									
D	43 to 56	Operating near capacity	Near capacity & accident study required									
E	57 to 70	At capacity; at signals, incidents will cause excessive delays; Roundabouts require other control mode	At capacity, requires other control mode									

Source: RTA (2002) Guide to Traffic Generating Developments



Conclusions

• Proposed parking provision

- Complies with the requirements of State Environmental Planning Policy (Affordable Rental Housing) 2009 for car parking provision.
- Complies with Council's Development Control Plan requirements in terms of design.
- Traffic impacts
 - The additional traffic from the proposed development will be minimal and will have no negative impacts on street network operation
- Design of access, car parking and servicing facilities
 - Complies with the relevant Standards
- The proposed development is supportable on traffic and parking grounds.

Fundad

Oleg I. Sannikov Director MEngSc (Traffic Engineering) MIEAust, PEng FAITPM



References:

State Environmental Planning Policy (Affordable Rental Housing) 2009 Liverpool Development Control Plan 2008 Guide to Traffic Generating Developments RMS (2002) AS/NZS 2890.1:2004: Parking Facilities – Off-street car parking AS/NZS 2890.6:2009: Parking Facilities – Off-street parking for people with disabilities



Appendix

Bus routes Results of SIDRA analysis Car park design checks and vehicle turning diagrams

Bus Route 851



Bus Route 852



Bus Route 865



Bus Route 866



SITE LAYOUT

Site: 1 [Kurrajong Rd / Ingham Dr AM Ex]

18052 Kurrajong Rd / Ingham Dr AM Ex Site Category: (None) Roundabout

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V Site: 1 [Kurrajong Rd / Ingham Dr AM Ex]

18052 Kurrajong Rd / Ingham Dr AM Ex Site Category: (None) Roundabout

Mover	Movement Performance - Vehicles												
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South:	Ingham D	r											
1	L2	189	1.7	0.594	10.4	LOS A	5.7	40.2	0.84	0.91	0.98	41.5	
3	R2	288	1.8	0.594	13.3	LOS A	5.7	40.2	0.84	0.91	0.98	44.4	
Approa	ach	478	1.8	0.594	12.2	LOS A	5.7	40.2	0.84	0.91	0.98	43.3	
East: K	(urrajong F	Rd (East)											
4	L2	262	2.0	0.669	8.0	LOS A	7.4	53.3	0.78	0.72	0.82	47.3	
5	T1	429	4.9	0.669	8.0	LOS A	7.4	53.3	0.78	0.72	0.82	22.3	
Approa	ach	692	3.8	0.669	8.0	LOS A	7.4	53.3	0.78	0.72	0.82	33.6	
West: I	Kurrajong I	Rd (West)											
11	T1	381	6.6	0.647	8.3	LOS A	6.9	50.0	0.81	0.80	0.89	33.8	
12	R2	234	0.5	0.647	11.1	LOS A	6.9	50.0	0.81	0.80	0.89	46.0	
Approa	ach	615	4.3	0.647	9.4	LOS A	6.9	50.0	0.81	0.80	0.89	40.3	
All Veh	icles	1784	3.4	0.669	9.6	LOS A	7.4	53.3	0.80	0.80	0.89	38.7	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 1 [Kurrajong Rd / Ingham Dr AM Fu]

18052 Kurrajong Rd / Ingham Dr AM Fu Site Category: (None) Roundabout

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand F Total veh/h	lows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South:	Ingham D	r												
1	L2	191	1.7	0.597	10.5	LOS A	5.7	40.6	0.85	0.91	0.98	41.4		
3	R2	288	1.8	0.597	13.3	LOS A	5.7	40.6	0.85	0.91	0.98	44.4		
Approa	ach	479	1.8	0.597	12.2	LOS A	5.7	40.6	0.85	0.91	0.98	43.2		
East: K	Currajong F	Rd (East)												
4	L2	262	2.0	0.672	8.1	LOS A	7.5	54.1	0.78	0.73	0.83	47.2		
5	T1	431	4.9	0.672	8.1	LOS A	7.5	54.1	0.78	0.73	0.83	22.3		
Approa	ach	693	3.8	0.672	8.1	LOS A	7.5	54.1	0.78	0.73	0.83	33.5		
West: I	Kurrajong	Rd (West)												
11	T1	385	6.6	0.653	8.4	LOS A	7.1	51.2	0.81	0.80	0.90	33.7		
12	R2	236	0.4	0.653	11.2	LOS A	7.1	51.2	0.81	0.80	0.90	45.9		
Approa	ach	621	4.2	0.653	9.4	LOS A	7.1	51.2	0.81	0.80	0.90	40.2		
All Veh	icles	1793	3.4	0.672	9.7	LOS A	7.5	54.1	0.81	0.80	0.90	38.6		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 1 [Kurrajong Rd / Ingham Dr PM Ex]

18052 Kurrajong Rd / Ingham Dr PM Ex Site Category: (None) Roundabout

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South:	Ingham D	r											
1	L2	122	1.7	0.616	10.9	LOS A	6.2	43.6	0.87	0.93	1.03	40.8	
3	R2	368	1.1	0.616	13.7	LOS A	6.2	43.6	0.87	0.93	1.03	43.8	
Approa	ach	491	1.3	0.616	13.0	LOS A	6.2	43.6	0.87	0.93	1.03	43.1	
East: K	Currajong F	Rd (East)											
4	L2	294	1.4	0.709	8.6	LOS A	8.8	63.2	0.83	0.75	0.91	46.9	
5	T1	439	3.6	0.709	8.6	LOS A	8.8	63.2	0.83	0.75	0.91	22.1	
Approa	ach	733	2.7	0.709	8.6	LOS A	8.8	63.2	0.83	0.75	0.91	33.8	
West: I	Kurrajong I	Rd (West)											
11	T1	396	3.5	0.724	11.1	LOS A	9.2	65.7	0.92	0.95	1.15	30.6	
12	R2	237	0.0	0.724	14.0	LOS A	9.2	65.7	0.92	0.95	1.15	43.4	
Approa	ach	633	2.2	0.724	12.2	LOS A	9.2	65.7	0.92	0.95	1.15	37.2	
All Veh	icles	1856	2.2	0.724	11.0	LOS A	9.2	65.7	0.87	0.87	1.02	37.8	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 1 [Kurrajong Rd / Ingham Dr PM Fu]

18052 Kurrajong Rd / Ingham Dr PM Fu Site Category: (None) Roundabout

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South:	Ingham D	r											
1	L2	123	1.7	0.620	11.0	LOS A	6.2	44.2	0.87	0.94	1.04	40.7	
3	R2	368	1.1	0.620	13.8	LOS A	6.2	44.2	0.87	0.94	1.04	43.7	
Approa	ach	492	1.3	0.620	13.1	LOS A	6.2	44.2	0.87	0.94	1.04	43.0	
East: K	Currajong F	Rd (East)											
4	L2	294	1.4	0.713	8.7	LOS A	9.0	64.3	0.84	0.75	0.92	46.9	
5	T1	442	3.6	0.713	8.7	LOS A	9.0	64.3	0.84	0.75	0.92	22.0	
Approa	ach	736	2.7	0.713	8.7	LOS A	9.0	64.3	0.84	0.75	0.92	33.8	
West: I	Kurrajong I	Rd (West)											
11	T1	397	3.4	0.726	11.2	LOS A	9.3	66.3	0.93	0.95	1.15	30.5	
12	R2	238	0.0	0.726	14.1	LOS A	9.3	66.3	0.93	0.95	1.15	43.3	
Approa	ach	635	2.2	0.726	12.3	LOS A	9.3	66.3	0.93	0.95	1.15	37.2	
All Veh	icles	1862	2.1	0.726	11.1	LOS A	9.3	66.3	0.88	0.87	1.03	37.8	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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SITE LAYOUT

✓ Site: 2 [Kurrajong Rd / Tallowwood Ave AM Ex]

18052 Kurrajong Rd / Tallowwood Ave AM Ex Site Category: (None) Giveway / Yield (Two-Way)



Kurrajong Rd (East)

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V Site: 2 [Kurrajong Rd / Tallowwood Ave AM Ex]

18052 Kurrajong Rd / Tallowwood Ave AM Ex Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
East: k	Kurrajong	Rd (East)											
5	T1	561	4.5	0.311	0.3	LOS A	0.4	3.0	0.08	0.03	0.09	58.9	
6	R2	23	0.0	0.311	8.9	LOS A	0.4	3.0	0.08	0.03	0.09	43.2	
Approach		584	4.3	0.311	0.7	NA	0.4	3.0	0.08	0.03	0.09	58.7	
North:	Tallowwo	od Ave											
7	L2	38	0.0	0.097	6.1	LOS A	0.3	2.3	0.58	0.76	0.58	25.8	
9	R2	19	0.0	0.097	13.0	LOS A	0.3	2.3	0.58	0.76	0.58	45.8	
Approa	ach	57	0.0	0.097	8.4	LOS A	0.3	2.3	0.58	0.76	0.58	36.5	
West:	Kurrajong	Rd (West))										
10	L2	38	72.2	0.306	6.4	LOS A	0.0	0.0	0.00	0.04	0.00	48.2	
11	T1	559	0.0	0.306	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	59.8	
Approa	ach	597	4.6	0.306	0.4	NA	0.0	0.0	0.00	0.04	0.00	58.9	
All Veh	nicles	1238	4.3	0.311	0.9	NA	0.4	3.0	0.06	0.06	0.07	58.0	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 2 [Kurrajong Rd / Tallowwood Ave AM Fu]

18052 Kurrajong Rd / Tallowwood Ave AM Fu Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
East: k	Kurrajong	Rd (East)											
5	T1	561	4.5	0.313	0.3	LOS A	0.4	3.3	0.09	0.03	0.10	58.8	
6	R2	25	0.0	0.313	9.0	LOS A	0.4	3.3	0.09	0.03	0.10	43.0	
Approach		586	4.3	0.313	0.7	NA	0.4	3.3	0.09	0.03	0.10	58.6	
North:	Tallowwo	od Ave											
7	L2	44	0.0	0.114	6.1	LOS A	0.4	2.7	0.58	0.77	0.58	25.7	
9	R2	22	0.0	0.114	13.1	LOS A	0.4	2.7	0.58	0.77	0.58	45.7	
Approa	ach	66	0.0	0.114	8.5	LOS A	0.4	2.7	0.58	0.77	0.58	36.4	
West:	Kurrajong	Rd (West)	1										
10	L2	39	70.3	0.307	6.4	LOS A	0.0	0.0	0.00	0.04	0.00	48.3	
11	T1	559	0.0	0.307	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	59.7	
Approa	ach	598	4.6	0.307	0.4	NA	0.0	0.0	0.00	0.04	0.00	58.9	
All Veh	nicles	1251	4.2	0.313	1.0	NA	0.4	3.3	0.07	0.07	0.08	57.8	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 3 [Kurrajong Rd / Tallowwood Ave PM Ex]

18052 Tallowwood / Ironbark Ave Ave AM Ex Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
East: k	Kurrajong	Rd (East)											
5	T1	580	3.1	0.337	0.6	LOS A	0.8	5.4	0.13	0.04	0.16	58.1	
6	R2	38	2.8	0.337	9.5	LOS A	0.8	5.4	0.13	0.04	0.16	41.3	
Approa	ach	618	3.1	0.337	1.1	NA	0.8	5.4	0.13	0.04	0.16	57.8	
North:	Tallowwoo	od Ave											
7	L2	26	8.0	0.077	6.5	LOS A	0.3	1.9	0.61	0.77	0.61	24.4	
9	R2	13	8.3	0.077	15.2	LOS B	0.3	1.9	0.61	0.77	0.61	44.4	
Approa	ach	39	8.1	0.077	9.3	LOS A	0.3	1.9	0.61	0.77	0.61	34.7	
West:	Kurrajong	Rd (West)											
10	L2	45	30.2	0.318	5.9	LOS A	0.0	0.0	0.00	0.04	0.00	48.8	
11	T1	584	0.0	0.318	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	59.4	
Approa	ach	629	2.2	0.318	0.5	NA	0.0	0.0	0.00	0.04	0.00	58.6	
All Veh	nicles	1286	2.8	0.337	1.1	NA	0.8	5.4	0.08	0.06	0.10	57.6	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: TEF Consulting | Processed: Monday, 11 June 2018 22:26:56

V Site: 3 [Kurrajong Rd / Tallowwood Ave PM Fu]

18052 Tallowwood / Ironbark Ave Ave AM Fu Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
East: k	Kurrajong	Rd (East)											
5	T1	580	3.1	0.340	0.7	LOS A	0.8	5.9	0.14	0.04	0.17	58.0	
6	R2	41	2.6	0.340	9.5	LOS A	0.8	5.9	0.14	0.04	0.17	41.0	
Approa	ach	621	3.1	0.340	1.2	NA	0.8	5.9	0.14	0.04	0.17	57.6	
North:	Tallowwo	od Ave											
7	L2	28	7.4	0.083	6.5	LOS A	0.3	2.0	0.61	0.78	0.61	24.4	
9	R2	14	7.7	0.083	15.2	LOS B	0.3	2.0	0.61	0.78	0.61	44.4	
Approa	ach	42	7.5	0.083	9.3	LOS A	0.3	2.0	0.61	0.78	0.61	34.8	
West:	Kurrajong	Rd (West)	l .										
10	L2	48	28.3	0.319	5.9	LOS A	0.0	0.0	0.00	0.04	0.00	48.8	
11	T1	584	0.0	0.319	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	59.4	
Approa	ach	633	2.2	0.319	0.5	NA	0.0	0.0	0.00	0.04	0.00	58.5	
All Veh	nicles	1296	2.8	0.340	1.1	NA	0.8	5.9	0.09	0.07	0.10	57.4	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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SITE LAYOUT

∇ Site: 3 [Tallowwood Ave / Ironbark Ave AM Ex]

18052 Tallowwood Ave / Ironbark Ave AM Ex Site Category: (None) Giveway / Yield (Two-Way)



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V Site: 3 [Tallowwood Ave / Ironbark Ave AM Ex]

18052 Tallowwood Ave / Ironbark Ave AM Ex Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South:	Tallowwo	od Ave (So	uth)										
10	L2	14	0.0	0.014	3.8	LOS A	0.0	0.0	0.00	0.27	0.00	48.9	
11	T1	15	0.0	0.014	0.0	LOS A	0.0	0.0	0.00	0.27	0.00	53.9	
Approach		28	0.0	0.014	1.8	NA	0.0	0.0	0.00	0.27	0.00	51.4	
North: Tallowwood		od Ave (No	rth)										
5	T1	24	0.0	0.013	0.0	LOS A	0.0	0.0	0.01	0.03	0.01	59.0	
6	R2	1	0.0	0.013	5.5	LOS A	0.0	0.0	0.01	0.03	0.01	54.3	
Approa	ach	25	0.0	0.013	0.2	NA	0.0	0.0	0.01	0.03	0.01	58.7	
West:	Ironbark A	Ave											
7	L2	1	0.0	0.022	5.6	LOS A	0.1	0.5	0.10	0.56	0.10	46.2	
9	R2	26	0.0	0.022	5.6	LOS A	0.1	0.5	0.10	0.56	0.10	38.9	
Approa	ach	27	0.0	0.022	5.6	LOS A	0.1	0.5	0.10	0.56	0.10	39.3	
All Veh	nicles	81	0.0	0.022	2.6	NA	0.1	0.5	0.04	0.29	0.04	48.4	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 3 [Tallowwood Ave / Ironbark Ave AM Fu]

18052 Tallowwood Ave / Ironbark Ave AM Fu Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South:	Tallowwo	od Ave (So	uth)										
10	L2	17	0.0	0.016	3.8	LOS A	0.0	0.0	0.00	0.30	0.00	48.4	
11	T1	15	0.0	0.016	0.0	LOS A	0.0	0.0	0.00	0.30	0.00	53.3	
Approach		32	0.0	0.016	2.0	NA	0.0	0.0	0.00	0.30	0.00	50.7	
North: Tallowwood		od Ave (No	rth)										
5	T1	24	0.0	0.013	0.0	LOS A	0.0	0.0	0.01	0.03	0.01	59.0	
6	R2	1	0.0	0.013	5.5	LOS A	0.0	0.0	0.01	0.03	0.01	54.3	
Approa	ach	25	0.0	0.013	0.2	NA	0.0	0.0	0.01	0.03	0.01	58.7	
West:	Ironbark A	Ave											
7	L2	1	0.0	0.030	5.6	LOS A	0.1	0.7	0.11	0.56	0.11	46.2	
9	R2	36	0.0	0.030	5.6	LOS A	0.1	0.7	0.11	0.56	0.11	38.9	
Approa	ach	37	0.0	0.030	5.6	LOS A	0.1	0.7	0.11	0.56	0.11	39.2	
All Veh	nicles	94	0.0	0.030	3.0	NA	0.1	0.7	0.04	0.33	0.04	47.1	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 3 [Tallowwood Ave / Ironbark Ave PM Ex]

18052 Tallowwood Ave / Ironbark Ave PM Ex Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Tallowwood Ave (South)														
10	L2	38	2.8	0.032	3.8	LOS A	0.0	0.0	0.00	0.34	0.00	47.0		
11	T1	24	0.0	0.032	0.0	LOS A	0.0	0.0	0.00	0.34	0.00	52.6		
Approach		62	1.7	0.032	2.3	NA	0.0	0.0	0.00	0.34	0.00	49.1		
North:	Tallowwo	od Ave (No	orth)											
5	T1	11	10.0	0.006	0.0	LOS A	0.0	0.0	0.03	0.06	0.03	57.6		
6	R2	1	0.0	0.006	5.6	LOS A	0.0	0.0	0.03	0.06	0.03	53.5		
Approa	ach	12	9.1	0.006	0.5	NA	0.0	0.0	0.03	0.06	0.03	57.0		
West:	Ironbark A	Ave												
7	L2	1	0.0	0.024	5.6	LOS A	0.1	0.6	0.12	0.56	0.12	46.1		
9	R2	27	7.7	0.024	5.7	LOS A	0.1	0.6	0.12	0.56	0.12	38.6		
Approa	ach	28	7.4	0.024	5.7	LOS A	0.1	0.6	0.12	0.56	0.12	39.0		
All Veh	nicles	102	4.1	0.032	3.1	NA	0.1	0.6	0.04	0.37	0.04	46.5		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 3 [Tallowwood Ave / Ironbark Ave PM Fu]

18052 Tallowwood Ave / Ironbark Ave PM Fu Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South:	Tallowwo	od Ave (So	outh)										
10	L2	44	2.4	0.035	3.8	LOS A	0.0	0.0	0.00	0.36	0.00	46.8	
11	T1	24	0.0	0.035	0.0	LOS A	0.0	0.0	0.00	0.36	0.00	52.2	
Approach		68	1.5	0.035	2.5	NA	0.0	0.0	0.00	0.36	0.00	48.6	
North:	Tallowwo	od Ave (No	orth)										
5	T1	11	10.0	0.006	0.0	LOS A	0.0	0.0	0.03	0.06	0.03	57.5	
6	R2	1	0.0	0.006	5.6	LOS A	0.0	0.0	0.03	0.06	0.03	53.5	
Approa	ach	12	9.1	0.006	0.5	NA	0.0	0.0	0.03	0.06	0.03	57.0	
West:	ronbark A	ve											
7	L2	1	0.0	0.026	5.6	LOS A	0.1	0.6	0.12	0.56	0.12	46.1	
9	R2	31	6.9	0.026	5.8	LOS A	0.1	0.6	0.12	0.56	0.12	38.6	
Approa	ach	32	6.7	0.026	5.7	LOS A	0.1	0.6	0.12	0.56	0.12	38.9	
All Veh	icles	112	3.8	0.035	3.2	NA	0.1	0.6	0.04	0.39	0.04	46.1	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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